

PERMISSIBLE LOAD ON THE BASE OF GROUND DAMS AT SEISMIC IMPACTS

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Abstract

The basis of this report is the issue of assessing the amount of permissible load on the ground of earthen dams in the seismic process. As a result of the research, an expression for determining the maximum load capacity of loose soils was proposed. In the report the question of definition of size of a maximum load on the basis of dams in seismic conditions of their work is considered. On the basis of the accepted settlement scheme the formula for maximum load calculation with reference to soil, not possessing forces of connectivity is offered.

Keywords: loess soil. water-saturated, seismic subsidence, subsidence, clayey soils, laying depth.

The stability of the foundation of any structure in order to avoid possible severe consequences must be ensured in all cases. But construction practice shows that even the fulfillment of this requirement alone does not always guarantee the strength of structures and normal conditions for their operation.

Especially often, such a situation occurs when there are soils such as moistened loess and loess-like rocks in the thickness of the base of structures.

Seismic subsidence of structures for a number of reasons (heterogeneity of the structure of the base and the properties of the foundations that make it up, the unevenness of the dynamic load and stress distribution in the soil layer, etc.) is always uneven to one degree or another. In many cases, it is measured in tens of centimeters and more than 1.0 meters in the presence of loess deposits with an unstable structure at the base.

Under such conditions, there is a danger of a violation of the strength of the structure with the

manifestation of cracks and ruptures in it as a result of overvoltages that have arisen due to their deformation. Consequently, in most cases, when considering issues of ensuring the stability of foundation soils, one of the most reliable methods is to limit the value of the ultimate load on the foundation of structures.

Let us conditionally assume that the structure was erected on the surface of the soil layer, and we will enter into the calculation the mass of the soil layer with a thickness of h_3 . In this case, the design load on the soil at the level of the base of the foundation is determined from the condition:

$$p_p = p_0 (1 + k_c e^{i\omega t}) - \gamma h_3 \quad (1)$$

where p_0 is the pressure from the weight of the structure;
 k_c seismicity coefficient.

Thus, taking into account the deepening of a structure is reduced to considering the following condition:

$$P_h = \gamma z = \gamma (z + h_3) \quad (2)$$

where h_3 is the depth of the considered horizon below the level of the actual application of the load to the soil, i.e. below the base of the foundation.

Let us assume that at a depth h from the soil surface, the base of the foundation of a structure with an area F that transfers pressure to the soil is laid. It is required to find the foundation depth h , at which the oscillating foundation will not give deformation (seismic subsidence). Let us replace the structure with a column of soil, with height h , having the same base area F and weight. Consequently, the pressure transmitted by the soil column to the base will be the same as that transmitted by this structure. Denoting the soil density as before, we have:

$$HF\gamma = p_0 \quad (3)$$

or taking into account the forces of inertia:

$$HF\gamma = p_0 (1 + k_c e^{i\omega t}) \quad (4)$$

Let us assume that a prism has broken off from a column of soil along the plane AB with a basesoles, and due to the destruction of the soil under its base AF , it tends to settle into the ground.

Destruction of soil stability under the foundation is represented as a certain prism AFC (collapse prism) tending to slide down the AC plane. This shift will be prevented by the FCD prism (resistance prism) and the soil layer $GFDE$ loaded on it, with a height h . Obviously, the collapse prism will press along the plane FC on the resistance prism with some force Q .

Let us assume that the force required to shift the resistance prism and the layer of soil lying on it will be equal to R and the force Q is directed normally to the surface FC . The angle value is determined by the assumption that R will be minimal.

The soil stability condition will therefore be defined as

$$R > Q \quad (5)$$

Obviously, the pressure Q will be active and the pressure R passive. The angles of inclination are defined as:

$$\alpha = 45^\circ - \varphi/2 \quad (6)$$

$$\beta = 45^\circ + \varphi/2 \quad (7)$$

The values of Q and R in accordance with the equations of the theory of soil pressure on the retaining wall will be equal to: для активного давления:

$$Q = (1/2 \gamma z^2 + \gamma H z) \left[\operatorname{tg} \right]^2 (45^\circ - \varphi/2), \quad (8)$$

for passive:

$$R = (1/2 \gamma z^2 + \gamma h z) \operatorname{tg}^2 (45^\circ + \varphi/2) \quad (9)$$

Since, according to the limiting condition of dynamic stability, $R = Q$, then, substituting their values from expressions (8) and (9), after the appropriate transformation, we obtain:

$$p_0 (1+k_c e^{\gamma h}) \operatorname{tg}^2 (45^\circ-\varphi/2)=\gamma h \operatorname{tg}^2 (45^\circ+\varphi/2) \quad (10)$$

or

$$\gamma h=p_0 (1+k_c e^{\gamma h})\left(\frac{\operatorname{tg}^2 (45^\circ-\varphi/2)}{\operatorname{tg}^2 (45^\circ+\varphi/2)}\right) \quad (11)$$

In expressions (10) and (11) means the pressure from the structure, equal to the height of the soil prism h , exerting the same pressure on the base with its weight. However, given that:

$$1/\left(\operatorname{tg}^2 (45^\circ+\varphi/2)\right)=1/\operatorname{ctg}(45^\circ-\varphi/2)=\operatorname{tg}^2 (45^\circ-\varphi/2) \quad (12)$$

expression (11) for the desired load can be represented as follows:

$$P_{np}=(H\gamma(1+k_c e^{\gamma h}))/\left(\operatorname{tg}^4 (45^\circ-\varphi/2)\right) \quad (13)$$

Expression (13) has one very remarkable property. As follows from its consideration, the value is directly proportional to H .

Formula (13) is the main expression for determining the ultimate load on the foundation of a structure, taking into account the seismic impact.

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